

#### 3.7.1 SEISMIC DESIGN PARAMETERS

#### **REVIEW RESPONSIBILITIES**

Primary - Structural Civil Engineering and Geosciences Branch (ESGBECGB<sup>1</sup>)

Secondary - None

#### I. AREAS OF REVIEW

The following areas relating to seismic design parameters are reviewed:

#### 1. <u>Design Ground Motion</u>

For the seismic design of nuclear power plants, it is customary to specify the earthquake ground motion that is exerted on the structure or on the soil-structure interacting system. The design ground motion, sometimes known as the seismic input or control motion, is based on the seismicity and geologic conditions at the site and expressed in such a manner that it can be applied to the dynamic analysis of structures. The three components of the design ground motions for the operating basis earthquake (OBE) and safe shutdown earthquake (SSE) are reviewed. They should be consistent with the description of the free-field ground motion at the site provided in SRP Section 2.5.2, which includes the variation in and distribution of ground motion in the free field, sources and directions of motion, propagation and transmission of seismic waves, and other site response characteristics.

The control motion should be defined to be on a free ground surface and should be based on data obtained in the free field (see NUREG/CP-0054, Reference .-17)<sup>2</sup>. Two cases are identified depending on the soil characteristics at the site and subject to availability of appropriate recorded ground-motion data. When data are available, for example, for relatively uniform sites of soil or

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#### **USNRC STANDARD REVIEW PLAN**

Standard review plans are prepared for the guidance of the Office of Nuclear Reactor Regulation staff responsible for the review of applications to construct and operate nuclear power plants. These documents are made available to the public as part of the Commission's policy to inform the nuclear industry and the general public of regulatory procedures and policies. Standard review plans are not substitutes for regulatory guides or the Commission's regulations and compliance with them is not required. The standard review plan sections are keyed to the Standard Format and Content of Safety Analysis Reports for Nuclear Power Plants. Not all sections of the Standard Format have a corresponding review plan.

Published standard review plans will be revised periodically, as appropriate, to accommodate comments and to reflect new information and experience.

Comments and suggestions for improvement will be considered and should be sent to the U.S. Nuclear Regulatory Commission, Office of Nuclear Reactor Regulation, Washington, D.C. 20555.

rock with smooth variation of properties with depth, the control point (location at which the control motion is applied) should be specified on the soil surface at the top of the finished grade. The free-field ground motion or control motion should be consistent with the properties of the soil profile. For sites composed of one or more thin soil layers overlying a competent material or in case of insufficient recorded ground-motion data, the control point is specified on an outcrop or a hypothetical outcrop at a location on the top of the competent material. The control motion specified should be consistent with the properties of the competent material. (Note: This information is reviewed under the review responsibility of SRP Section 2.5.2.)

#### a. <u>Design Response Spectra</u>

Design response spectra are used in the design of seismic Category I structures, systems, and components (SSCs)<sup>3</sup>, and they are generally specified in the free field. The proposed design response spectra for the operating basis earthquake (OBE)<sup>4</sup> and<sup>5</sup> safe shutdown earthquake (SSE)<sup>6</sup> (Ref. 2), as defined in Appendix A to 10 CFR 100,<sup>7</sup> are reviewed against the free-field response spectra that are reviewed and accepted under SRP Section 2.5.2. The distinction between the free-field response spectra (site specific or otherwise) reviewed in Section 2.5.2 and the design response spectra reviewed in this section is that a design response spectrum, in general, is a relatively smooth plot not exhibiting random peaks and valleys while a free-field response spectrum may exhibit random sharp peaks and valleys. The use of smooth spectra is preferred in the design and in certain situations it is needed (for example, in the design of a standard plant). The use of unsmoothed response spectra will be reviewed on a case-by-case basis.

## b. <u>Design Time History</u>

For computing the response of Category I structures and equipments, acceleration time histories may be used. These time histories must be compatible with the design response spectra, site specific or otherwise, as described above.

When an appropriate recorded or specified time history is not available as input motion for seismic system analysis, an artificial time history (three components) may be generated from the design response spectra for the purpose of carrying out a time history analysis. The response spectra obtained from such an artificial time history of motion should generally envelop the design response spectra for all damping values to be used. The procedures used to generate response spectra are reviewed.

In addition to the comparison of the response spectra derived from the time history with the design response spectra, the frequency intervals at which the spectral values are calculated are also reviewed. When time history analyses are performed, either of the following options may be considered. In either case the time histories may be real or artificial.

#### Option 1: Single Time History

Use of single time history is justified by satisfying a target power spectral density (PSD) requirement in addition to the design response spectra enveloping requirements.

#### Option 2: Multiple Time Histories

In lieu of the use of a single time history, multiple artificial or real time histories may be used for analyses and design of structures, systems, and components SSCs<sup>8</sup>. The number and adequacy of time histories with respect to design response spectra are reviewed.

In some instances, a nonlinear analysis of the structures, systems, and components SSCs<sup>9</sup> may be appropriate (e.g., the evaluation of existing structures). Multiple time history analyses incorporating real earthquake time histories are appropriate when such analyses are proposed. The adequacy of time histories used in the analyses is reviewed.

# 2. <u>Percentage of Critical Damping Values</u>

The percentage of critical damping values used for Category Istructures, systems, and components SSCs<sup>10</sup> is reviewed for both the OBE and the SSE. Critical damping is the amount of damping that would completely eliminate free vibration and is an important measure of the damping capacity of a structure.

Vibrating structures have energy losses that depend on numerous factors, such as material characteristics, stress levels, and geometric configuration. This dissipation of energy, or damping effect, occurs because a part of the excitation input is transformed into heat, sound waves, and other energy forms. The response of a system to dynamic loads is a function of the amount and type of damping existing in the system. A knowledge of appropriate values to represent this characteristic is essential for obtaining realistic results in dynamic analysis.

In practical seismic analysis, which usually employs linear methods of analysis, damping is also used to account for many nonlinear effects such as changes in boundary conditions, joint slippage, concrete cracking, gaps, and other effects that tend to alter response amplitude. In real structures, it is often impossible to separate "true" material damping from system damping, which is the measure of the energy dissipation, from the nonlinear effects. Overall structural damping used in design is normally determined by observing experimentally the total response of the structure.

Only the overall damping used for Category I-structures, systems, and components SSCs<sup>11</sup> is reviewed. When applicable, the basis for any damping values that differ from those given in Regulatory Guide 1.61(Ref. 3)<sup>12</sup> is reviewed.

### 3. <u>Supporting Media for Category I Structures</u>

The description of the supporting media for each Category I structure is reviewed, including foundation embedment depth, depth of soil over bedrock, soil layering characteristics, design groundwater elevation, dimensions of the structural foundation, total structural height, and soil properties to permit evaluation of the applicability of finite-element or lumped-spring approaches for soil-structure interaction analysis.

#### 4. Interface of Review Interfaces<sup>13</sup>

The ECGB also performs the following reviews under the SRP section indicated: 14

- 1.<sup>15</sup> Review of geological and seismological information to establish the free-field ground motion is performed as described in SRP Sections 2.5.1 through 2.5.3.
- 2. The geotechnical parameters and methods employed in the analysis of free field soil media and the soil properties are reviewed as described in SRP section 2.5.4.

The results of the reviews for the <del>operating basis earthquake (OBE)</del><sup>17</sup> and the <del>safe shutdown earthquake (SSE)</del><sup>18</sup> site-dependent free-field ground motion, soil properties, etc., are used as an integral part of the seismic analysis review of Category I structures.<sup>19</sup>

For those areas of review identified above as being part of other SRP sections, the acceptance criteria necessary for the review and their methods of application are contained in the referenced SRP sections.

### II. <u>ACCEPTANCE CRITERIA</u>

The staff accepts the design of structures that are important to safety and that must withstand the effects of the earthquakes if the relevant requirements of General Design Criterion (GDC) 2 (Ref. 4)<sup>20</sup> and Appendix A to 10 CFR Part 100 (Ref. 2)<sup>21</sup> concerning natural phenomena are complied with. The relevant requirements of GDC 2 and Appendix A to 10 CFR Part 100 are:

- +A.<sup>22</sup> For GDC 2 The design basis shall reflect appropriate consideration of the most severe earthquakes that have been historically reported for the site and surrounding area with sufficient margin for the limited accuracy, quantity, and period of time in which historical data have been accumulated.
- 2B.<sup>23</sup> For Appendix A to 10 CFR Part 100<sup>24</sup> Two earthquake levels, the safe shutdown earthquake (SSE)<sup>25</sup> and the operating basis earthquake (OBE)<sup>26</sup>, shall be considered in the design of the safety-related structures, components, and systems.<sup>27</sup>

Specific criteria necessary to meet the relevant requirements of GDC 2 and Appendix A to 10 CFR Part 100are described below. The acceptance criteria for the areas of review described in subsection I above are as follows: 128

# 1. <u>Design Ground Motion</u>

#### a. <u>Design Response Spectra</u>

The proposed OBE and SSE design response spectra for use in analyses and design of structures, systems, and components SSCs<sup>29</sup> should, generally, meet or exceed amplitudes of the site-specific spectra at all frequencies. The use of generic spectra, such as Regulatory Guide 1.60-(Ref. 5)<sup>30</sup> spectra, as design spectra is also acceptable provided that their use is consistent with the information reviewed in SRP Section 2.5.2.

To be acceptable, the design response spectra should be specified for three mutually orthogonal directions - two horizontal and one vertical. Current practice is to assume that the maximum ground accelerations in the two horizontal directions are equal.

### b. <u>Design Time History</u>

The SSE and OBE design time histories to be used in the free field of the soil media shall be consistent with those developed or specified in Section 2.5.2. For both horizontal and vertical input motions, either a single time history or multiple time histories can be used.

For linear structural analyses, using site-independent response spectra (e.g., Regulatory Guide 1.60), the total duration of the artificial accelerogram should be long enough such that adequate representation of the Fourier components at low frequency is included in the time history. The total time duration between 10 seconds and 25 seconds is required to adequately match the design response spectra at 0.4 Hz. The corresponding stationary phase strong-motion duration should be between 6 seconds and 15 seconds. If site-specific information reviewed in SRP Section 2.5.2 indicates duration estimates outside the above ranges, the site-specific values should be used. The rationale for selecting lower and upper limits on duration is presented in NUREG/CR-5347 (Reference 610)<sup>31</sup>. For nonlinear problems, duration estimates are reviewed on a case-by-case basis.

#### Option 1: Single Time History

To be considered acceptable, the response spectra of the artificial time history to be used in the free field must envelop the free-field design response spectra for all damping values actually used in the response analysis.

When spectral values are calculated from the artificial time history, the frequency intervals at which spectral values are determined are to be small enough such that

any reduction in these intervals does not result in more than 10 percent change in the computed spectra.

Table 3.7.1-1 provides an acceptable set of frequencies at which the response spectra may be calculated. Another acceptable method is to choose a set of frequencies such that each frequency is within 10 percent of the previous one.

Table 3.7.1-1 Suggested Frequency Intervals for Calculation of Response Spectra

Frequency Range (hertz)	Increment (hertz)
0.2 - 3.0	0.10
3.0 - 3.6	0.15
3.6 - 5.0	0.20
5.0 - 8.0	0.25
8.0 - 15.0	0.50
15.0 - 18.0	1.0
18.0 - 22.0	2.0
22.0 - 34.0	3.0

Each calculated spectrum of the artificial time history is considered to envelop the design response spectrum when no more than five points fall below, and no more than 10 percent below, the design response spectrum.

Recent studies indicate that numerically generated artificial ground acceleration histories produce power spectral density (PSD) functions having a quite different appearance from one individual function to another, even when all these time histories are generated so as to closely envelop the same design response spectra. For example, the use of the available techniques of generating acceleration histories that satisfy enveloping Regulatory Guide 1.60 (Ref. 5)<sup>32</sup> spectra usually results in PSD functions that fluctuate significantly and randomly as a function of frequency. It is also recognized that the more closely one tries to envelop the specified design response spectra, the more significantly and randomly do the spectral density functions tend to fluctuate and these fluctuations may lead to an unconservative estimate of response of some-structures, systems, and components SSCs<sup>33</sup>. Therefore, when a single artificial time history is used in the design of seismic Category I-structures, systems, and/or components SSCs<sup>34</sup>, it must in

general satisfy requirements for both enveloping design response spectra as well as adequately matching a target PSD function compatible with the design response spectra (see NUREG/CR-3509, Reference .79)<sup>35</sup>. Therefore, in addition to the response spectra enveloping requirement, the use of a single time history will also be justified by demonstrating sufficient energy at the frequencies of interest through the generation of PSD function, which is greater than a target PSD function throughout the frequency range of significance.

When Regulatory Guide 1.60 spectra are used as design spectra the requirements for a compatible target PSD are contained in Appendix A to this SRP section.<sup>36</sup> Target PSD functions other than those given in Appendix A can be used if justified. For site-specific design response spectra or spectra other than Regulatory Guide 1.60 spectra, a compatible target PSD should be generated. For generation of target PSD in such cases, procedures outlined in NUREG/CR-5347<del>Reference 6</del><sup>37</sup> can be used. For cases where a time history ensemble is used for generation of site-specific spectra, the same time histories can be used to generate mean plus one standard deviation (or 84th percentile) PSD function as a target PSD function. Procedures used to generate the target PSD will be reviewed on a case-by-case basis. The PSD requirements are included as secondary and minimum requirements to prevent potential deficiency of power over the frequency range of interest. It should be noted that the ground motion is still primarily defined by the design response spectrum. The use of PSD criteria itselfalone<sup>38</sup> can yield time histories that may not envelop the design response spectrum.

#### Option 2: Multiple Time Histories

As discussed in Section I.1.b of this SRP, the use of multiple real or artificial time histories for analyses and design of structures, systems, and components SSCs<sup>39</sup> is acceptable. As a minimum, four time histories should be used for analyses. Any lesser number will be reviewed and accepted on a case-by-case basis.

The parameters describing the time histories and the calculated response spectra for each time history are reviewed. The response spectra calculated for each individual time history need not envelop the design response spectra. However, the multiple time histories are acceptable if the average calculated response spectra generated from these time histories envelop the design response spectra. The design response spectra are considered to be the mean plus one standard deviation (or 84th percentile) response spectra as defined in Section 2.5.2.

The review of the real time histories used in the nonlinear analysis is conducted on a case-by-case basis. Some of the specific items of interest are number of time histories, frequency content, amplitude, energy content, duration, number of strong-motion cycles, and the basis for selection of time histories.

Additional information on the use of multiple time histories can be found in NUREG/CR-1161 (Reference 88)<sup>40</sup>. This information may be used for reference only, as it does not constitute the staff's acceptance criteria.

# 2. <u>Percentage of Critical Damping Values</u>

The specific percentage of critical damping values used in the analyses of Category I structures, systems, and components SSCs<sup>41</sup> are considered to be acceptable if they are in accordance with Regulatory Guide 1.61 (Ref. 3)<sup>42</sup>. Alternatively, damping values derived from Appendix N to ASME Code Section III, Division 1, Code Case N-411-1, insofar as they are compatible with the guidance provided in Regulatory Guide 1.84, are acceptable.<sup>43</sup> Higher damping values may be used in a dynamic seismic analysis if test data are provided to support them. These values will be reviewed and accepted by the staff on a case-by-case basis.

In addition, a demonstration of the correlation between stress levels and damping values will be required and reviewed for compliance with regulatory position C.3 of Regulatory Guide 1.61. Methods for correlation of damping values with stress level are discussed in NUREG/CR-1161References 8<sup>44</sup> and 9in ASCE Std 4-86 (Reference 11)<sup>45</sup>. If such methods are used, they will be reviewed and accepted on a case-by-case basis.

The damping values for foundation soils must be based upon measured values or other pertinent laboratory data, considering variation in soil properties and strains within the soil, and must include an evaluation of dissipation from pore pressure effects as well as material damping for saturated site conditions.

### 3. <u>Supporting Media for Category I Structures</u>

To be acceptable, the description of supporting media for each Category I structure must include foundation embedment depth, depth of soil over bedrock, soil layering characteristics, design groundwater elevation, dimensions of the structural foundation, total structural height, and soil properties such as shear wave velocity, shear modulus, Poisson's ratios, and density as a function of depth.

### Technical Rationale:46

The technical rationale for application of the above acceptance criteria to this Seismic Design Parameter section is discussed in the following paragraphs.<sup>47</sup>

1. General Design Criterion 2 requires, in relevant part, that SSCs important to safety be designed to withstand the effects of natural phenomena such as earthquakes without loss of capability to perform their intended safety functions. GDC 2 further requires that the design bases reflect appropriate consideration for the most severe natural phenomena that have been historically reported for the site and surrounding area, with sufficient margin for the limited accuracy, quantity, and period of time in which the historical data have been accumulated in the past. This SRP section reviews seismic design parameters to assure that they are appropriate and contain sufficient margin such that seismic analyses

(reviewed under other SRP sections) accurately and/or conservatively represent the behavior of SSCs during postulated seismic events. Applicable guidance for implementation of and achieving compliance with the requirements set forth in General Design Criterion 2 is provided in Regulatory Guides 1.60 and 1.61. Regulatory Guide 1.60 provides a procedure that is acceptable to the staff for defining response spectra for input into the seismic design analysis of nuclear power plants. Additionally, Regulatory Guide 1.61 delineates damping values acceptable to the staff, to be used in performing dynamic seismic analysis of Seismic Category I SSCs. Meeting the requirements of General Design Criterion 2, in conjunction with the guidelines provided in Regulatory Guides 1.60 and 1.61 assures that safety-related SSCs will continue to function following a seismic event, such that the plant can be brought to, and maintained in, a safe shutdown condition.

2. Appendix A to 10 CFR 100 requires that either a suitable dynamic analysis or an appropriate qualification test be used to demonstrate that all SSCs important to safety are capable of withstanding the seismic and other concurrent loads, except where it can be demonstrated that the use of an equivalent static load methodology provides adequate conservatism. The requirements of Appendix A to 10 CFR 100 assure that the applicable levels of vibratory ground motion corresponding to the OBE and the SSE are properly defined and that adequate accuracy and conservatism is being applied in defining the parameter values being used for input into the seismic design analysis. Compliance with the requirements detailed in Appendix A to 10 CFR 100 in conjunction with implementation of the requirements provided in General Design Criterion 2, as discussed above, assures that safety-related SSCs will continue to function following a seismic event, such that the plant can be brought to, and maintained in, a safe shutdown condition.

#### III. REVIEW PROCEDURES

For each area of review, the following review procedure is followed. The reviewer will select and emphasize material from the procedures given below as may be appropriate for a particular case. The scope and depth of review procedures must be such that the acceptance criteria described above are met.

#### 1. Design Ground Motion

#### a. Design Response Spectra

Design response spectra for the OBE and SSE for all damping values are checked to ensure that the spectra are in accordance with the acceptance criteria as given in subsection II. Any differences between the free-field spectra and the proposed design response spectra that have not been adequately justified are identified, and the applicant is informed of the need for additional technical justification.

### b. <u>Design Time History</u>

Methods of defining the design time histories are reviewed to confirm that the acceptance criteria of subsection II.2 of this SRP section are met.

# 2. <u>Percentage of Critical Damping Values</u>

The specific percentage of critical damping values for the OBE and SSE used in the analyses of Category I structures, systems, and components SSCs<sup>48</sup> are checked to ensure that the damping values are in accordance with the acceptance criteria as given in subsection II.2 of this SRP section. Any differences in damping values that have not been adequately justified are identified, and the applicant is informed of the need for additional technical justification.

### 3. <u>Supporting Media for Category I Structures</u>

The description of the supporting media is reviewed to verify that sufficient information, as specified in the acceptance criteria of subsection II.3 of this SRP section, is included. Any deficiency in the required information is identified, and a request for additional information is transmitted to the applicant.

In the ABWR and System 80+ design certification FSERs the staff accepted an exemption from the 10 CFR 100 Appendix A requirement that all safety-related SSCs be designed to remain functional and within applicable stress and deformation limits when subjected to an OBE. This exemption was based on alternative analyses performed by the licensees based only on the SSE and additional procedural requirements to perform an inspection of the plant following an earthquake at or above one-third the SSE. The Staff concluded that the alternative analysis accomplished the design objectives of the OBE design analysis and that the plant inspection requirements met the Commission-approved Staff recommendations discussed in SECY 93-087 (Reference 6).<sup>49</sup>

For standard design certification reviews under 10 CFR Part 52, the procedures above should be followed, as modified by the procedures in SRP Section 14.3 (proposed), to verify that the design set forth in the standard safety analysis report, including inspections, tests, analysis, and acceptance criteria (ITAAC), site interface requirements and combined license action items, meet the acceptance criteria given in subsection II. SRP Section 14.3 (proposed) contains procedures for the review of certified design material (CDM) for the standard design, including the site parameters, interface criteria, and ITAAC.<sup>50</sup>

#### IV. EVALUATION FINDINGS

The reviewer verifies that sufficient information has been provided and that histhe<sup>51</sup> evaluation supports conclusions of the following type, to be included in the staff's safety evaluation report.

The staff concludes that the seismic design parameters used in the plant structure design are acceptable and meet the requirements of General Design Criterion 2 and Appendix A to 10 CFR Part 100. This conclusion is based on the following.

The applicant has met the relevant requirements of GDC 2 and Appendix A to 10 CFR Part 100 by appropriate consideration for the most severe earthquake recorded for the site with an appropriate margin and considerations for two levels of earthquakes - the safe shutdown earthquake (SSE) and operating basis earthquake (OBE). The applicant has met these requirements by the use of the methods and procedures indicated below.

The seismic design response spectra (OBE and SSE) applied in the design of seismic Category I structures, systems, and components (SSCs)<sup>52</sup> meets or exceeds<sup>53</sup> the free-field response spectra provided in SRP<sup>54</sup> Section 2.5.2. The percentage of critical damping values used in the seismic analysis of Category I structures, systems, and components SSCs<sup>55</sup> are in conformance with Regulatory Guide 1.61, "Damping Values for Seismic Analysis of Nuclear Power Plants." The artificial time history used for seismic design of Category I plant structures, systems, and components SSCs<sup>56</sup> is adjusted in amplitude and frequency content to obtain response spectra that envelop the design response spectra specified for the site and also exhibits sufficient energy in the frequency range of interest. Conformance with the recommendations of Section 2.5.2 and Regulatory Guide 1.61 ensures that the seismic inputs to Category I structures, systems, and components SSCs<sup>57</sup> are adequately defined so as to form a conservative basis for the design of such structures, systems, and components SSCs<sup>58</sup> to withstand seismic loadings.

For design certification reviews, the findings will also summarize, to the extent that the review is not discussed in other safety evaluation report sections, the staff's evaluation of inspections, tests, analyses, and acceptance criteria (ITAAC), including design acceptance criteria (DAC), site interface requirements, and combined license action items that are relevant to this SRP section.<sup>59</sup>

#### V. IMPLEMENTATION

The following is intended to provide guidance to applicants and licensees regarding the NRC staff's plans for using this SRP section.

This SRP section will be used by the staff when performing safety evaluations of license applications submitted by applicants pursuant to 10 CFR 50 or 10 CFR 52.<sup>60</sup> Except in those cases in which the applicant proposes an acceptable alternative method for complying with specified portions of the Commission's regulations, the method described herein will be used by the staff in its evaluation of conformance with Commission regulations.

Implementation schedules for conformance to parts of the method discussed herein are contained in the referenced regulatory guides.

The provisions of this SRP section apply to reviews of applications docketed six months or more after the date of issuance of this SRP section.<sup>61</sup>

The provisions of this SRP section apply to reviews of construction permit (CP), preliminary design approval (PDA), final design approval (FDA), and combined license (CP/OL) applications docketed after the date of issuance of this SRP section. Operating license (OL) and final design approval (FDA) applications, whose CP and PDA reviews were conducted prior to the issuance of this rRevision 2 (dated August 1989) to SRP Section 3.7.1, will be reviewed in

accordance with the acceptance criteria given in the SRP Section 3.7.1, Revision 1, dated July 1981.<sup>63</sup>

#### VI. REFERENCES

- 41.64 10 CFR Part 50, Appendix A, General Design Criterion 2, "Design Bases for Protection Against Natural Phenomena."
- 2. 10 CFR Part 100, Appendix A, "Seismic and Geologic Siting Criteria for Nuclear Power Plants."
- 53. Regulatory Guide 1.60, "Design Response Spectra for Nuclear Power Plants."
- 34. Regulatory Guide 1.61, "Damping Values for Seismic Analysis for Nuclear Power Plants."
- 5. Regulatory Guide 1.84, "Design and Fabrication Code Case Acceptability, ASME Section III, Division 1." 65
- SECY-93-087, "Policy, Technical, and Licensing Issues Pertaining to Evolutionary and Advanced Light-Water Reactor (ALWR) Designs," April 2, 1993; SRM-93-087 issued on July 21, 1993.<sup>66</sup>
- 17. NUREG/CP-0054, "Proceedings of the Workshop on Soil-Structure Interaction," Bethesda, Maryland, NUREG/CP-0054, 67 June 16-18, 1986.
- 88. NUREG/CR-1161, D. W. Coats, "Recommended Revisions to Nuclear Regulatory Commission Seismic Design Criteria," NUREG/CR-1161, 68 May 1980.
- 79. NUREG/CR-3509, M. Shinozuka, T. Mochio, and E. F. Samaras, "Power Spectral Density Functions Compatible with NRC Regulatory Guide 1.60 Response Spectra," NUREG/CR-3509, 69 June 1988.
- 610. NUREG/CR-5347, A. J. Philippacopoulos, "Recommendations for Resolution of Public Comments on USI A-40, Seismic Design Criteria," NUREG/CR-5347, April June 1989.
- 911. ASCE Standard 4-86, "Seismic Analysis of Safety-Related Nuclear Structures and Commentary on Standard for Seismic Analysis of Safety-Related Nuclear Structures," American Society of Civil Engineers, September 1986.
- 12. ASME Boiler and Pressure Vessel Code, Section III, "Rules for Construction of Nuclear Power Plant Components," Division 1, Appendix N, "Dynamic Analysis Methods," American Society of Mechanical Engineers.<sup>72</sup>

# APPENDIX A TO SRP SECTION 3.7.1 SPECIFICATION FOR MINIMUM POWER SPECTRAL DENSITY (PSD) REQUIREMENT

For a Regulatory Guide 1.60 horizontal response spectrum anchored to 1.0 g, the following minimum PSD requirement should be satisfied. For other peak accelerations, this PSD requirement should be scaled by the square of the peak acceleration.

The onesided PSD is related to the Fourier amplitude  $|F(\omega)|^{73}$  of the time history by the equation

$$S_0(\omega) = \frac{2|F(\omega)|^2}{2\pi T_D}$$
 .....(1)

where  $T_D$  is the strong motion duration over which  $F(\omega)$  is evaluated. This duration  $T_D$  represents the duration of near maximum and nearly stationary power of an acceleration time history record. Additional guidance on estimation of  $T_D$  for artificial time history or actual earthquake time history is provided in Appendix B of NUREG/CR-5347 (Reference 610)<sup>74</sup>.

The average onesided PSD defined by Equation (1) should exceed 80 percent of the target PSD as defined by Equation (2) from 0.3 Hz to 24 Hz.

Less than 2.5 Hz

2.5 Hz to 9.0 Hz

$$S_0(\omega)(f) = 0.419 \text{ m}^2 \frac{650 \text{ inch}^2}{100} r^{8} \sec^3 (2.5 \text{ Hz/f})^{1.8} \dots (2)$$

9.0 Hz to 16.0 Hz

$$S_0(\omega)(f) = 418 \text{ cm}^2 \frac{64.8 \text{ inch}^2}{\text{sec}^3} (9.0 \text{ Hz/f})^3$$

Greater than 16 Hz

$$S_0(\omega)(f) = 74.2 \text{ cm}^2 \frac{11.5 \text{ inch}^2}{\text{sec}^3} (16.0 \text{ Hz/f})^8$$

At any frequency f, the average PSD is computed over a frequency band width of  $\pm 20$  percent, centered on the frequency f (e.g., 4 Hz to 6 Hz band width for f = 5 Hz).

The power above 24 Hz for the target PSD is so low as to be inconsequential so that checks above 24 Hz are unnecessary. (However, note that the response spectrum calculations are required beyond 24 Hz as governed by Regulatory Guide 1.60 definitions.) Similarly, power below 0.3 Hz has no influence on stiff nuclear plant facilities, so that checks below 0.3 Hz are

unnecessary. This minimum check is set at 80 percent of the target PSD so as to be sufficiently high to prevent a deficiency of power over any broad frequency band, but sufficiently low that this requirement introduces no additional conservatism over that already embodied in the Regulatory Guide 1.60 response spectrum.

A time history meeting this minimum PSD requirement will produce a response spectrum that lies below the Regulatory Guide 1.60 response spectrum at all frequencies. To produce a response spectrum that accurately fits the 2 percent damped, 1.0 g, Regulatory Guide 1.60 response spectrum at all frequencies above 0.25 Hz, the PSD defined by Equation (2) can be used with the resulting time history being clipped at  $\pm 1.0$  g (Ref. 7see NUREG/CR-3509, Reference 9)<sup>79</sup>.

To produce a response spectrum that conservatively envelops the 1.0 g Regulatory Guide 1.60 response spectrum at 2 percent damping and greater, a PSD set at 130 percent of the PSD defined by Equation (2) can be used with the resulting time history being clipped at  $\pm 1.0$  g.

# **SRP Draft Section 3.7.1**

# Attachment A - Proposed Changes in Order of Occurrence

Item numbers in the following table correspond to superscript numbers in the redline/strikeout copy of the draft SRP section.

Item	Source	Description
1.	SRP-UDP Format Item, Update PRB names.	Changed PRB name to reflect latest responsibility assignments for SRP section 3.7.1.
2.	Editorial.	Added document title and changed reference citation to the updated Item number in VI. References.
3.	Editorial.	Consistent with other SRP sections, added the acronym SSC for "structure, system and component," to be used in lieu of the full expression.
4.	Editorial.	Deleted "operating basis earthquake," as the acronym "OBE" had been previously defined in this SRP section.
5.	Integrated Impacts # 115 and # 1221.	Rulemaking has been proposed that will allow elimination of the OBE as an analysis requirement from the design basis, if its peak ground acceleration (PGA) level is set at 1/3 or less of the PGA for the SSE. Because this rulemaking is not yet final, no SRP change is being made at this time.
6.	Editorial.	Deleted "safe shutdown earthquake," as the acronym "SSE" had been previously defined in this SRP section.
7.	Editorial, SRP-UDP format item.	Deleted numbered reference citation as this type of citation is not to be used when quoting parts of the Code of Federal Regulations, including General Design Criteria, and also Regulatory Guides. Inserted document title in lieu of numbered reference citation.
8.	Editorial.	Substituted the acronym SSC for "structure, system and component." This acronym was introduced at the beginning of this SRP section, and its use is consistent with and common in other SRP sections.
9.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
10.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
11.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
12.	SRP-UDP format item.	Deleted numbered reference citation as this type of citation is not to be used when quoting parts of the Code of Federal Regulations, including General Design Criteria, and also Regulatory Guides.

Item	Source	Description
13.	SRP-UDP format item, Reformat Areas of Review.	Added "Review Interfaces" heading to Areas of Review. Reformatted existing description of review interfaces in numbered format to describe how the ECGB reviews aspects of the seismic system analysis under other SRP Sections and how other branches support the review. Review Interfaces is a new subsection.
14.	SRP-UDP format item, Reformat Areas of Review.	Added standard lead-in sentence for new "Review Interfaces" subsection.
15.	SRP-UDP format item, Reformat Areas of Review.	Segregated the existing paragraph into three separate items, consistent with the SRP-UDP guidelines for the new Review Interfaces subsection.
16.	SRP-UDP format item, Reformat Areas of Review.	Segregated the existing material into separate items, consistent with the SRP-UDP guidelines for the new Review Interfaces subsection.
17.	Editorial.	Deleted "operating basis earthquake," as the acronym "OBE" had been previously defined in this SRP section.
18.	Editorial.	Deleted "safe shutdown earthquake," as the acronym "SSE" had been previously defined in this SRP section.
19.	SRP-UDP format item, Reformat Areas of Review.	Segregated the existing paragraph into separate items, consistent with the SRP-UDP guidelines for the new Review Interfaces subsection.
20.	SRP-UDP format item.	Deleted numbered reference citation as this type of citation is not to be used when quoting parts of the Code of Federal Regulations, including General Design Criteria, and also Regulatory Guides.
21.	SRP-UDP format item.	Deleted numbered reference citation as this type of citation is not to be used when quoting parts of the Code of Federal Regulations, including General Design Criteria, and also Regulatory Guides.
22.	Editorial.	Changed numbering format from numerical to alphabetical format for general acceptance criteria to distinguish them from specific criteria, as numerical numbering format is being used for specific acceptance criteria.
23.	Editorial.	Changed numbering format from numerical to alphabetical format for general acceptance criteria to distinguish them from specific criteria, as numerical numbering format is being used for specific acceptance criteria.

Item	Source	Description
24.	Integrated Impact # 1221.	Rulemaking has been proposed in 59 FR 52255 that will lead to amendments in 10CFR100 with regard to, among other items, seismic requirements. However, because this rulemaking is not yet final, no SRP change is being made at this time, based on this Integrated Impact.
25.	Editorial.	Deleted "safe shutdown earthquake," as the acronym "SSE" had been previously defined in this SRP section.
26.	Editorial.	Deleted "operating basis earthquake," as the acronym "OBE" had been previously defined in this SRP section.
27.	Integrated Impacts # 115 and # 1221.	Rulemaking has been proposed that will allow elimination of the OBE as an analysis requirement from the design basis, if its peak ground acceleration (PGA) level is set at 1/3 or less of the PGA for the SSE. Because this rulemaking is not yet final, no SRP change is being made at this time.
28.	Editorial.	Because of partial overlap, combined two sentences into one, to eliminate partly duplicative wording and to make this paragraph more consistent with other SRP sections. Additionally, changed period at the end of the sentence to colon, for enhanced clarity.
29.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
30.	SRP-UDP format item.	Deleted numbered reference citation as this type of citation is not to be used when quoting parts of the Code of Federal Regulations, including General Design Criteria, and also Regulatory Guides.
31.	SRP-UDP format item, Editorial.	Added document title and changed reference citation to the updated Item number in VI. References.
32.	SRP-UDP format item.	Deleted numbered reference citation as this type of citation is not to be used when quoting parts of the Code of Federal Regulations, including General Design Criteria, and also Regulatory Guides.
33.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
34.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
35.	Editorial.	Added document title and changed reference citation to the updated Item number in VI. References.

Item	Source	Description
36.	Editorial.	Added missing document designation.
37.	Editorial.	Added numerical NUREG/CR document title in lieu of simple numerical reference citation. Parenthetical numerical reference citation is not needed here as this document has been cited previously in this SRP section with a parenthetical numerical reference citation.
38.	Editorial.	Altered wording, for clarification.
39.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
40.	Editorial.	Added document title and changed reference citation to the updated Item number in VI. References.
41.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
42.	SRP-UDP format item.	Deleted numbered reference citation as this type of citation is not to be used when quoting parts of the Code of Federal Regulations, including General Design Criteria, and also Regulatory Guides.
43.	Integrated Impact # 116.	Added the discussion of damping values derived from ASME Code Case N-411-1, as far as this Code Case is endorsed by Regulatory Guide 1.84, to Acceptance Criteria.
44.	Editorial.	Added numerical NUREG/CR document title in lieu of simple numerical reference citation. Parenthetical numerical reference citation is not needed here as this document has been cited previously in this SRP section with a parenthetical numerical reference citation.
45.	Editorial.	Added document titles and changed reference citations to the updated Item numbers in VI. References.
46.	SRP-UDP Format Item, Develop Technical Rationale.	Added Technical Rationale for General Design Criterion GDC 2 and for Appendix A to 10CFR100. Technical Rationale is a new feature, being added to the SRP.
47.	SRP-UDP Format Item, develop Technical Rationale.	Added standard leader sentence for new "Technical Rationale" subsection.
48.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.

Item	Source	Description
49.	Integrated Impact # 1399.	Added a discussion of the elimination of OBE from seismic design requirement that was accepted in the evolutionary FSERs.
50.	SRP-UDP Guidance, Implementation of 10 CFR 52	Added standard paragraph to address application of Review Procedures in design certification reviews.
51.	Editorial.	Changed "his" to "the," to improve gender neutrality of the SRP.
52.	Editorial.	Added the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
53.	Editorial.	Corrected verb forms, consistent with plural subject "spectra."
54.	Editorial.	Added the acronym "SRP," for clarification.
55.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
56.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
57.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
58.	Editorial.	Substituted the acronym SSC for "structure, system and component," as is consistent with the remainder of the SRP section.
59.	SRP-UDP Format Item, Implement 10 CFR 52 Related Changes	To address design certification reviews a new paragraph was added to the end of the Evaluation Findings. This paragraph addresses design certification specific items including ITAAC, DAC, site interface requirements, and combined license action items.
60.	SRP-UDP Guidance, Implementation of 10 CFR 52	Added standard sentence to address application of the SRP section to reviews of applications filed under 10 CFR Part 52, as well as Part 50.
61.	SRP-UDP Guidance	Added standard paragraph to indicate applicability of this section to reviews of future applications.
62.	SRP-UDP Guidance	Replaced the first sentence with a standard paragraph to indicate applicability of this section to reviews of future applications.
63.	Editorial.	Specified applicable revision numbers for SRP Section 3.7.1, to be used during the specified periods of time, accordingly.

Item	Source	Description
64.	Editorial.	Resequenced items in reference list to conform with currently used pattern: grouped by regulatory importance, and chronologically and/or in numerical order.
65.	Integrated Impact # 116.	Added Regulatory Guide 1.84 as a reference.
66.	Integrated Impact # 1399.	Added SECY-93-087 as a reference to this SRP section.
67.	SRP-UDP format item.	Changed wording of reference slightly, for consistency with generally used format for NUREG/CR reports.
68.	SRP-UDP format item.	Changed wording of reference slightly, for consistency with generally used format for NUREG/CR reports.
69.	SRP-UDP format item.	Changed wording of reference slightly, for consistency with generally used format for NUREG/CR reports.
70.	SRP-UDP format item.	Changed wording of reference slightly, for consistency with generally used format for NUREG/CR reports.
71.	Editorial and/or reference verification.	Corrected publishing date for NUREG/CR-5347. The manuscript for NUREG/CR-5347 was completed in February of 1989, and the report was published in June of 1989.
72.	Integrated Impact # 116.	Added Appendix N to ASME Code Section III, Division 1, as a reference, in connection with Regulatory Guide 1.84, which discusses the damping values of Code Case N-411-1.
73.	Editorial.	Corrected typographical error. Changed the argument, $(\omega)$ , of the function F, from subscripted to normal font. To avoid confusion in the formula notation, the subscripted $(\omega)$ was deleted rather than shown in strikeout font. The same correction has also been applied to the following equation (1). Note that the source material shows the correct notation; see, for instance, equation (11) on Page 10 of NUREG/CR-3509.
74.	Editorial.	Added document title and changed reference citation to the updated Item number in VI. References.
75.	Editorial.	Substituted the correct mathematical symbol, as the power spectral density profile specified in equation (2) is formulated as a function of frequency f in Hertz, not as a function of circular frequency $\omega = 2  \pi  f$ , which has the dimension of rad/s.
76.	SRP-UDP update item, metrication policy, Federal Standard 376B	Converted values from (inch) <sup>2</sup> to (m) <sup>2</sup> , pursuant to federal metrication policy as detailed in Std 376B.

Item	Source	Description
77.	Editorial and/or reference verification.	Added missing mathematical symbol which is required to assure that the expression in parentheses becomes dimensionless. The original record of these equations (see NUREG/CR-5347, Page B-8) also contains these symbols.
78.	Editorial.	Added missing slash in equation.
79.	Editorial.	Added document title and changed reference citation to the updated Item number in VI. References.

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# **SRP Draft Section 3.7.1**Attachment B - Cross Reference of Integrated Impacts

Integrated Impact No.	Issue	SRP Subsections Affected
115	Revise Areas of Review, Acceptance Criteria, and Review Procedures to reflect that the staff will consider elimination of the OBE from the design basis, on a case by case basis, as an exemption from 10 CFR 100.	No SRP change due to this ROC, pending regulatory rulemaking. However, an explanatory note is provided in II. Acceptance Criteria, Item 2, concerning Appendix A to 10 CFR 100.
116	Revise Areas of Review and Acceptance Criteria to allow usage of ASME Code Case N-411 damping values, in conjunction with Regulatory Guide 1.84.	II. Acceptance Criteria, Item 2; VI. References, Items 5 and 11.
117	Consider qualified endorsement of ASCE Std 4-86 (Seismic Analysis of Safety Related Nuclear Structures) as a candidate for future work.	No SRP change due to this ROC. This item will be tracked with IPD-7.0 Form 3.7.2-1.
1137	Revise Acceptance Criteria, Review Procedures, and Evaluation Findings, as necessary, to incorporate the guidance of the proposed Draft Regulatory Guide DG-1015.	No SRP change due to this ROC, pending final approval of the Draft Regulatory Guide.
1221	Revise the SRP to incorporate the new and revised requirements from proposed rulemaking 59 FR 52255.	No SRP change due to this ROC, pending final approval of changes to 10 CFR 100, 10 CFR 50.34, and of new Appendix S to 10 CFR 50.
1399	Revise the Review Procedures to provide information regarding the staff's past consideration of an applicant's proposal to eliminate or reduce the OBE as an exemption to 10 CFR 100.	III and VI